



GRS QC Flooring





Typical specification

Product Information

QC Flooring is a composite steel and concrete slab system for which the QC panels provide the following features:

- Shutter supporting the mass of the wet concrete and construction loads
- Tensile reinforcement resulting in a composite section with the concrete
- Finished ceiling ready for painting with PVA compound

QC panels are easy to erect and the sytem is adaptable to steel, concrete and brick structures. Provision can easily be made for electrical and mechanical services.

Product Specifications

Protection, storage and hoisting of materials

- The floor units shall be protected against damage in transit to the job site.
- If site storage is necessary, steel floor units shall be stacked clear of the ground on supports at 1,5m centres and tilted slightly to help prevent the entrapment of water. The units shall be protected from the elements with a tarpaulin or equal, leaving ends open to provide air circulation. The General Contractor shall provide the storage area and protect the floor units against damage.

Preparation and concreting - for composite QC Flooring Construction

Base Preparation

Prior to concreting, the surface of the steel floor units shall be cleaned of all debris to the satisfaction of the Engineer. The units are to be free of oil before despatch from our factory.

Concrete

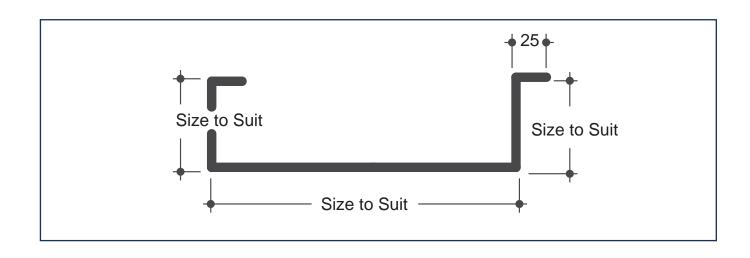
Unless otherwise stated by the Engineer, the concrete shall have a minimum cube compressive strength of 25MPa at 28 days. Materials, mixing, placing and curing of concrete shall be in accordance with SANS 10100-2: 1992, or in accordance with the Engineer's specification. Concrete used with QC panels shall not contain chloride salts or other deleterious material. No load shall be applied until the concrete has reached its required strength.

Reinforcement

Any additional reinforcement shall be supplied by the General Contractor and placed in accordance with the Engineer's specification and layout.



QC Flooring Profile



Typical specification

Propping

When required, and in accordance with Global Roofing Solution's specifications, composite QC Flooring systems shall be propped during construction. The propping shall be in true line, braced and strutted to prevent deformation under the mass of wet concrete and construction loads. The props shall be in accordance with good building practice. Removal of the props shall be in accordance with SANS 10100-2: 1992.

Installation

When fixing to steel construction, the floor units shall be placed on the supporting framework and adjusted to final position before permanent attachment. Each unit shall be brought to proper bearing on the supporting beams. If the supporting beams are not properly aligned or sufficiently level to permit proper bearing of the floor units, the erector shall bring the matter to the attention of the General Contractor for corrective action. The floor units shall be placed in straight alignment for the entire length of run of the flutes and laid in strict accordance with the manufacturer's instructions and as shown on a layout prepared for the erector's use. Side laps shall be properly engaged.

Design Information

QC as reinforcement only

Composite QC floor slabs are designed in the same manner as conventional reinforced concrete slabs. The steel of the panels is used as the tensile reinforcement; the bond with the concrete having been verified by tests.

QC with additional reinforcement

Additional reinforcement can be used for increased moment of resistance. This reinforcement acts in conjunction with the QC panels, increasing the bending moment of the composite section.

Propping

See tables 5 to 8.

Composite Slab Design

Calculations are based on the Limit State Method (SANS 10100-1: 2000).

Fastening

Side laps

The QC panel shall be joined together by means of clips at 500mm centres.

Steelwork

QC panels shall be fastened to the supporting steelwork by means of two self-tapping screws per unit at end bearing points.

Concrete / Brickwork

The QC panels shall be connected to each other with a steel strap placed across the panels and fixed to every upstand with poprivets or tek screws at each bearing point. This is done to ensure lateral stability while casting the slab.



Table 1

QC 54/250 1.0mm composite slab without reinforcement, propped during construction

				1.0m	nm Thick											
		Slab Depth (mm)	Dead Load KN/m ²		Maximum uniformly distributed superimposed load (including finishes) kN/m ²											
		` ′							Spa	an in me	tres					
				2.0	2.5	3.0	3.5	4.0	4.25	4.5	4.75	5.0	5.25	5.5	5.75	6.0
_	A	100	2.48	13.1	10.1	6.6	4.4	2.9	2.2	1.6						
		110	2.71	14.4	11.2	7.4	4.9	3.3	2.7	2.0						
)Ce		120	2.95	15.8	12.2	8.1	5.4	3.6	3.0	2.4	1.8					
Distance m		130	3.18	17.1	13.2	8.9	5.9	4.0	3.2	2.6	2.1	1.6				
		140	3.42	18.4	14.2	9.7	6.4	4.3	3.5	2.9	2.3	1.8				
ping 1,5		150	3.65	19.8	15.3	10.4	6.9	4.7	3.8	3.1	2.5	2.0	1.6			
Propping 1,50		160	3.89	21.1	16.3	11.2	7.4	5.0	4.1	3.3	2.7	2.1	1.7			
		170	4.12	22.4	17.3	11.9	7.9	5.3	4.4	3.6	2.9	2.3	1.8			
		180	4.36	23.7	18.3	12.7	8.4	5.7	4.7	3.8	3.1	2.5	1.9			
1,35 m	A	190	4.59	25.1	19.4	13.4	8.9	6.0	5.0	4.0	3.3	2.6	2.1	1.6		
1,3	١	200	4.83	26.4	20.4	14.2	9.4	6.4	5.2	4.3	3.5	2.8	2.2	1.7		

Loads in blue determined by bond.

Loads in yellow determined by moment of resistance.

Loads in green determined by deflection under total load (Span/250), or deflection under live load (Span/350) but ≤ 20mm.

Loads determined by bond or resistance can be increased by including reinforcement. An anti-crack fabric reinforcement is recommended in all cases (Ref 193).

Bold black line indicated maximum Span/Depth ratio at 30.

Table 2
QC 54/250 1.2mm composite slab without reinforcement, propped during construction

				1.2m	nm Thick														
	Slab Depth (mm)	Dead Load KN/m ²					Maximu	m unifo	rmly dis	tributed	superir	nposed	load (in	cluding	finishes	s) kN/m²	2		
									S	Span in n	netres								
				2.0	2.5	3.0	3.5	4.0	4.25	4.5	4.75	5.0	5.25	5.5	5.75	6.0	6.25	6.5	6.75
A	100	2.5		13.1	10.1	8.1	5.5	3.5	2.5	1.8									
	110	2.74		14.4	11.1	8.9	6.2	4.2	3.1	2.2	1.5								
	120	2.97		15.8	12.2	9.8	6.8	4.7	3.8	2.8	2.0								
	130	3.21		17.1	13.2	10.6	7.5	5.2	4.3	3.5	2.6	1.8							
	140	3.44		18.4	14.2	11.4	8.1	5.6	4.7	3.9	3.2	2.4	1.7						
	150	3.68		19.7	15.2	12.2	8.8	6.1	5.1	4.2	3.5	2.9	2.2	1.5					
_	160	3.91		21.1	16.3	13.1	9.4	6.5	5.4	4.5	3.8	3.1	2.6	2.1					
1,50 m	170	4.15		22.4	17.3	13.9	10.1	7.0	5.8	4.9	4.0	3.3	2.7	2.2	1.8				
۲,	180	4.38		23.7	18.3	14.7	10.7	7.4	6.2	5.2	4.3	3.6	2.9	2.4	1.9				
	190	4.62		25.0	19.3	15.5	11.4	7.9	6.6	5.5	4.6	3.8	3.1	2.5	2.0	1.6			
	200	4.85		26.4	20.4	16.4	12.0	8.3	7.0	5.8	4.9	4.0	3.3	2.7	2.2	1.7			
	210	5.09		27.7	21.4	17.2	12.7	8.8	7.4	6.1	5.1	4.3	3.5	2.9	2.3	1.8			
	220	5.33		29.0	22.4	18.0	13.3	9.2	7.7	6.5	5.4	4.5	3.7	3.0	2.4	1.9			
	230	5.56		30.3	23.4	18.8	13.9	9.7	8.1	6.8	5.7	4.7	3.9	3.2	2.5	2.0	1.5		
•	240	5.8		31.7	24.5	19.7	14.6	10.2	8.5	7.1	5.9	4.9	4.1	3.3	2.7	2.1	1.6		

Loads in blue determined by bond.

Loads in yellow determined by moment of resistance.

Loads in green determined by deflection under total load (Span/250), or deflection under live load (Span/350) but ≤ 20mm. Loads determined by bond or resistance can be increased by including reinforcement. An anti-crack fabric reinforcement is recommended in all cases (Ref 193).

Bold black line indicated maximum Span/Depth ratio at 30.

Transverse bowing likely to be exceeded

Transverse bowing ikely to be exceeded

QC 54/250 tables

Table 3
QC 54/250 1.6mm composite slab without reinforcement, propped during construction

			1.6m	nm Thick										
	Slab Depth (mm)	Dead Load KN/m ²		Maximum uniformly distributed superimposed load (including finishes) kN/m²										
								Sp	an in me	tres				
			3.0	3.5	4.0	4.5	5.0	5.5	6.0	6.5	7.0	7.5	8.0	8.5
_	160	3.96	13.0	10.7	9.0	6.8	5.0	3.6	2.2					
	170	4.2	13.9	11.4	9.6	7.3	5.3	3.9	2.6					
	180	4.43	14.7	12.1	10.2	7.8	5.7	4.1	3.0	1.7				
	190	4.67	15.5	12.8	10.7	8.3	6.1	4.4	3.1	2.1				
92	200	4.9	16.3	13.5	11.3	8.8	6.4	4.7	3.4	2.3				
star	210	5.14	17.1	14.1	11.9	9.3	6.8	5.0	3.6	2.5	1.6			
Propping Distance 1,60 m	220	5.38	18.0	14.8	12.5	9.8	7.2	5.2	3.7	2.6	1.7			
opin 1,	230	5.61	18.8	15.5	13.0	10.3	7.5	5.5	4.0	2.7	1.8			
Pro	240	5.85	19.6	16.2	13.6	10.8	7.9	5.8	4.1	2.9	1.9			
	260	6.32	21.3	17.5	14.8	11.8	8.6	6.3	4.6	3.2	2.1			
	280	6.79	22.9	18.9	15.9	12.8	9.4	6.9	5.0	3.5	2.3			
	300	7.26	24.6	20.3	17.1	13.8	10.1	7.4	5.4	3.8	2.5			
	320	7.73	26.2	21.6	18.2	14.7	10.8	8.0	5.8	4.0	2.7	1.6		

Loads in blue determined by bond.

Loads in yellow determined by moment of resistance.

Loads in green determined by deflection under total load (Span/250), or deflection under live load (Span/350) but ≤ 20mm. Loads determined by bond or resistance can be increased by including reinforcement. An anti-crack fabric reinforcement is recommended in all cases (Ref 193).

Bold black line indicated maximum Span/Depth ratio at 30.

Table 4
QC 54/250 2.0mm composite slab without reinforcement, propped during construction

			2.0m	ım Thick										
	Slab Depth (mm)	Dead Load KN/m ²		Maximum uniformly distributed superimposed load (including finishes) kN/m²										
	()	,					Sp	an in me	tres					
			4.0	4.5	5.0	5.5	6.0	6.5	7.0	7.5	8.0	8.5	9.0	
1	200	4.95	11.3	9.6	8.3	6.6	5.0	3.7	2.7	1.8				
	220	5.42	12.4	10.6	9.1	7.4	5.5	4.1	3.0	2.1				
	240	5.9	13.6	11.6	10.0	8.1	6.1	4.6	3.3	2.3	1.5			
JCe	260	6.37	14.7	12.6	10.8	8.9	6.7	5.0	3.7	2.6	1.7			
istai	280	6.84	15.9	13.5	11.7	9.7	7.3	5.5	4.0	2.8	1.9			
ng Dis 1,60 m	300	7.31	17.0	14.5	12.5	10.5	7.9	5.9	4.4	3.1	2.1			
Propping Distance 1,60 m	320	7.78	18.2	15.5	13.4	11.2	8.5	6.4	4.7	3.3	2.2			
Pro	340	8.25	19.3	16.5	14.2	12.0	9.1	6.8	5.1	3.6	2.4			
	360	8.72	20.5	17.5	15.1	12.8	9.7	7.3	5.4	3.9	2.6	1.6		
	380	9.19	21.6	18.4	15.9	13.6	10.3	7.8	5.7	4.1	2.8	1.7		
<u> </u>	400	9.66	22.8	19.4	16.8	14.3	10.9	8.2	6.1	4.4	3.0	1.8		

Loads in blue determined by bond.

Loads in yellow determined by moment of resistance.

Loads in green determined by deflection under total load (Span/250), or deflection under live load (Span/350) but \leq 20mm. Loads determined by bond or resistance can be increased by including reinforcement. An anti-crack fabric reinforcement is recommended in all cases (Ref 193).

Bold black line indicated maximum Span/Depth ratio at 30.

QC unpropped tables

Table 5

QC UNPROPPED: 1.0mm composite slab unpropped during concreting

QC Panel dimension DEPTH / WIDTH

Table 6

QC UNPROPPED: 1.2mm composite slab unpropped during concreting

QC Panel dimension DEPTH / WIDTH

Slab Depth	Depth Load		Span in metres									
(mm)	KN/m ²		1.5	2.0	2.5	3.0	3.5					
100	2.4					No	No					
110	2.64			60/235	75/205	INO	INO					
120	2.88					90/175						
130	3.12						105/145					
140	3.36		54/250	65/225	80/195							
150	3.6					95/165						
160	3.84				85/185							
170	4.08			70/215		100/155	No					
180	4.32											
190	4.56			75/205	90/175	105/145						
200	4.8		60/235									

Tables allow for construction loads according to S.A. Steel Construction handbook.

The load carried by the composite section is similar to that of a 54 / 250 section of the same thickness.

Slab Depth	Dead Load		Spar	n in metres			
(mm)	KN/m ²	1.5	2.0	2.5	3.0	3.5	4.0
100	2.4			60/235	75/205	No	
110	2.64		54/250			85/185	No
120	2.88			65/225			
130	3.12				80/195	90/175	100/155
140	3.36						105/145
150	3.6		60/235	70/215			
160	3.84				85/185	95/165	
170	4.08	54/250					
180	4.32					100/155	
190	4.56			75/205		100/155	No
200	4.8				90/175		INO
210	5.04					105/145	
220	5.28						
230	5.52		65/225	80/195			
240	5.76				No	No	
250	6		70/215	85/185			

Table 7

QC UNPROPPED: 1.6mm composite slab unpropped during concreting

QC Panel dimension DEPTH / WIDTH

Slab Depth	Dead Load				Span in	metres					
(mm)	KN/m ²	1.5	2.0	2.5	3.0	3.5	4.0	4.5	5.0	5.5	6.0
150	3.6				65/225			95/165	105/145		
160	3.84			60/235		80/195	90/175				
170	4.08				70/215			100/155			
180	4.32										
190	4.56					85/185					
200	4.8		54/250		75/205		95/165	105/145			
210	5.04										
220	5.28	54/250		65/225							
230	5.52					90/175	100/155				
240	5.76				80/195				No	No	No
250	6										
260	6.24						105/145	No			
270	6.48			70/215							
280	6.72		60/235		85/185	95/165					
290	6.96										
300	7.2										
310	7.44			75/205		No	No				
320	7.68				90/175	INU					



QC unpropped tables

Table 8

QC UNPROPPED: 2.0mm composite slab unpropped

during concreting

QC Panel dimension DEPTH / WIDTH

Slab Depth	Dead Load		Span in metres									
(mm)	KN/m ²	2.0	2.5	3.0	3.5	4.0	4.5	5.0	5.5	6.0	6.5	
200	4.8		54/250	65/225	75/205	85/185	95/165					
220	5.28		34/230	03/223	73/203	03/103	93/103					
240	5.76				00/405		100/155					
260	6.24		60/235	70/215	80/195	90/175	105/145					
280	6.72		00/235									
300	7.2	54/250			85/185	95/165		No	No	No	No	
320	7.68			75/205		100/155						
340	8.16		65/225				No					
360	8.64				90/175	105/145						
380	9.12		70/215	80/195								
400	9.6	60/235	10/215		95/165							



Composite slab design criteria

Calculations are based on the Limit State Method (SANS 10100-1: 2000)

- Cube strength of concrete at 28 days = 25 MPa
- Modulus of Elasticity of steel = 200 GPa
- Modular ratio for deflection calculations =

$$\frac{Es}{Ec} = 10$$

Commerical Quality Steel: Assumed Yield strength = 200 MPa

• Partial safety factors for strength (bond, shear,

bending)

 $\begin{array}{ll} \text{steel} & \text{ym} = 1,15 \\ \text{concrete} & \text{ym} = 1,5 \\ \text{dead load} & \text{yf} = 1,2 \\ \text{imposed load} & \text{yf} = 1,6 \\ \end{array}$

For serviceability, all factors equal one:

- Ultimate shear stress: see tables 5 and 14 of SABS 0100 of 1980.
- Ultimate bond stress: ratio between shear and lever arm = 0,41 kN/m per mm width based on SABS tests
- Minimum recommended slab depth (in mm) = t x 100, where it is the unit thickness in mm
- Resistance factor for bending and shear $\emptyset = 0.90$

Construction loads

A construction load of 1,5kN/m² or line load of 2kN/m, 300mm in width transverse to deck is added to the dead load of the wet concrete and unit, when determining the section modulus of the unit.

End bearing requirements

Minimum end bearing on to brick walls = 75mm Minimum end bearing on to reinforced concrete beams and structural steel beams = 50mm

Fire rating

QC panels in 1,0mm and 1,2mm have an SABS certified 2 hour fire rating for framed structures

Deflection limits

• Maximum deflection of the composite slab under total load: $\Delta \le \frac{\text{span}}{250}$

The limit does not take into account the initial deflection under wet concrete of propless units.

 Maximum deflection of the composite slab due to imposed load alone:

$$\Delta \leq \frac{\text{span or 20mm, whichever is smaller}}{350}$$

• Maximum deflection of propless units under wet concrete: $\Delta \leq \frac{\text{span}}{}$

• Transverse bowing (across unit width):

$$\Delta \leq \frac{\text{base width}}{75}$$

(unless otherwise specified in tables)

Sectional Properties

The design and evaluation of sectional properties of QC units is in accordance with BS 5950: Part 4. Code of practice for design of floors with profiled steel sheeting.

Mass Of QC panels										
Thickness (mm)	Mass (kg/m)									
1.0	3.27									
1.2	3.96									
1.6	5.32									
2.0	6.68									



Please visit our website or contact GRS for standard flashing details

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